

BALTIMORE COUNTY, MARYLAND  
DEPARTMENT OF PUBLIC WORKS & TRANSPORTATION  
DIVISION OF CONSTRUCTION CONTRACTS ADMINISTRATION  
111 WEST CHESAPEAKE AVENUE  
TOWSON, MARYLAND 21204



Contract No. 20203 WX0  
Towson Water Pumping Station Renovations –  
7781 Far Hills Drive, Towson, Maryland 21286  
Towson – District 9c5  
Job Order No. / Workday No.  
231-203-0035-0445 / 030350445

**ADDENDUM NO.6**

**DATE:** 10/27/2023

**Contact:** Anthony Crews, 410-887-3531, [tcrews@baltimorecountymd.gov](mailto:tcrews@baltimorecountymd.gov)

**To All Bidders**

This addendum is hereby made a part of the Proposal and the Special Provisions, and is hereby incorporated into the Contract. Should this addendum conflict with any portion of the Special Provisions, the Proposal, or any prior addenda, this addendum shall supersede and control.

Please note the attached changes, corrections, and/or information in connection with the contract and submit bids and be otherwise governed accordingly.

**For Your Information**

Attached is the Geotechnical Report and Soil Boring Logs.

Attachments – 36

**PLEASE SIGN BELOW ACKNOWLEDGING RECEIPT OF THIS  
ADDENDUM AND RETURN WITH YOUR BID.**

\_\_\_\_\_  
Company Name

\_\_\_\_\_  
Signature

**TOWSON FINISHED WATER RESERVOIR  
GENERATOR & SUBSTATION BUILDINGS**

**BALTIMORE, MARYLAND**

*Prepared for:*

**Gannett Fleming, Inc.**  
7133 Rutherford Road, Suite 300  
Baltimore, Maryland 21244

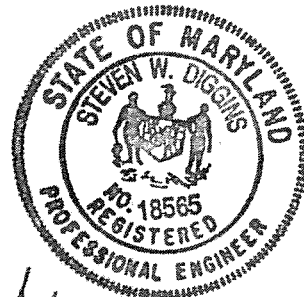
*Prepared by:*



**EBA Engineering, Inc.**  
4813 Seton Drive | Baltimore, MD 21215  
(410) 358-7171 | [www.ebaengineering.com](http://www.ebaengineering.com)

Project No. 3090-03

August 2013



Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.  
License No.: 18565, Expiration Date: 01/19/2014.

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## **Geotechnical Report**

### **Towson Finished Water Reservoir Generator and Substation Buildings Baltimore, Maryland**

#### **INTRODUCTION**

EBA Engineering, Inc. (EBA) was retained by Gannett Fleming, Inc. (Gannett Fleming) to perform a geotechnical investigation for the proposed improvements at the Towson Finished Water Reservoir in Baltimore, Maryland. Gannett Fleming is performing the design of the project through a contract with Baltimore City. The proposed improvements include the construction of emergency generator and substation buildings.

The purpose of the investigation was to determine the stratigraphy and engineering characteristics of the soils at the project site, and to provide recommendations for the design and construction of the proposed buildings in consideration of the subsurface conditions. This report presents a description of the investigation, summary of the subsurface conditions, and recommendations regarding the design and construction of foundations for the proposed emergency generator and substation buildings. The report appendices include a project vicinity map, soil boring location plan, boring logs and laboratory test results.

#### **SITE GEOLOGY**

The project site lies on the western edge of the transition zone between the Atlantic Coastal Plain and the Piedmont Plateau physiographic provinces, a boundary commonly referred to as the Fall Line or Fall Zone. The stratigraphy of the Fall Line zone is characterized by a relatively thin capping of Coastal Plain sediment and/or residual soils on the crystalline rocks of the Piedmont. The Maryland Geological Survey's map of the Towson Quadrangle (1974) indicates the presence of sediment at the project site consisting of the sand and gravel facies of the Patuxent Formation, a division of the Potomac Group of Cretaceous age. The sand and gravel facies consist of well sorted medium to fine grained quartz sand with locally abundant quartz gravel and clay clasts/layers. These facies are characterized by abrupt horizontal and vertical changes in lithology between the sand and gravel facies and the clay facies.

Generally, the bedrock in the Piedmont Plateau physiographic province has been weathered at the surface forming a relatively thin residuum of soil-like material. This residuum, including residual soil and decomposed rock, is the result of in-situ weathering of the parent bedrock. Residual soil is completely weathered and may not possess relict features of the parent rock. Decomposed rock is highly weathered, generally more coarse and rocky than residual soil and retains relict features of the parent rock such as coloring, discontinuities and bedding planes. Large cobble to boulder-sized fragments of rock are common in Piedmont soils. Piedmont soils generally become harder and denser with increasing depth.

## **PREVIOUS SUBSURFACE INVESTIGATION**

A previous subsurface investigation was conducted in June, 2006. The investigation included three soil borings, numbered BT-1, BT-2 and BT-12, which were performed in the vicinity of proposed generator building. These borings were advanced to depths ranging from 43.8 to 45.0 feet. Information from the previous subsurface investigation was reviewed and considered along with the data from the current investigation. The boring locations are shown on the Soil Boring Location Plan presented in Appendix B and boring logs are included in Appendix E.

## **SUBSURFACE EXPLORATION**

The subsurface exploration program consisted of two soil borings, numbered BT-13 and BT-14. The boring locations were staked in the field by EBA based on measurements to existing features shown on a plan provided by Gannett Fleming. The locations of underground utilities in the vicinity of each boring location were marked prior to performing the borings. The as-drilled location of each boring is shown on the Soil Boring Location Plan presented in Appendix B.

The borings were performed by MDA Drilling, Inc. (MDA) in August, 2013 under the direction of a geotechnical engineer from EBA. Borings BT-13 and BT-14 were advanced to a depth of 20 feet by the hollow stem auger drilling method using an ATV-mounted drilling rig. Standard Penetration Tests (SPT) were generally performed in the borings at 2.5-foot intervals to a depth of 10 feet and at 5-foot intervals thereafter. A soil sample was collected with a split barrel sampler at each SPT location.

The water level depth in each boring was measured upon completion and after 24 hours. Each boring was backfilled immediately after the final water level depth was recorded.

Descriptions of the soils encountered, SPT results, groundwater observations and other information are provided on the boring logs, which are presented in Appendix C. The ground surface elevations shown on the logs are based on the topographic information shown on a plan provided by Gannett Fleming. The soil descriptions provided on the logs were determined in general accordance with the procedures described in ASTM D2488 (ASTM International's version of the Unified Soil Classification System [USCS]).

## **LABORATORY TESTING**

The soil samples obtained from the borings were transported to EBA's laboratory, where selected representative samples were tested to determine the moisture contents, particle size distributions, and liquid and plastic limits of the soils. The results of the laboratory tests were used to determine certain engineering properties and to aid in the classification of the soils. Graphic plots and a tabular summary of the laboratory test results are provided in Appendix D.

## **SUBSURFACE CONDITIONS**

### **Topsoil**

Topsoil was encountered at the surface at the location of Boring BT-13. The thickness of the Topsoil was 5 inches as noted in the "Remarks" column of the boring log.

### **Existing Fill**

Existing fill was encountered at the surface at the location of Boring BT-14, and underlying the Topsoil in Boring BT-13. The depth to the bottom of this layer ranged from 6 to 8.5 feet. The Existing Fill consisted of medium stiff to very stiff, sandy CLAY and sandy SILT, and medium dense to very dense, silty SAND and GRAVEL with varying amounts of sand. Traces of roots and plastic pieces were observed in the samples. The SPT N-values in this layer ranged from 5 to more than 100 blows per foot and the average was 39 blows per foot.

The moisture content of one selected sample from this layer was 20.0 percent. The USCS symbols for the soils in this layer were found to include GM, SM, ML and CL and the AASHTO soil classification symbols were found to include A-1-b, A-2-4, A-4 and A-6 based on visual classifications.

### **Sandy Clay**

Sandy Clay was encountered underlying the Existing Fill in Boring BT-14. The depth to the bottom of this layer was 12 feet. This layer is believed to be associated with the Clay facies of Patuxent Formation. The Sandy Clay consisted of stiff to very stiff, sandy lean CLAY. The SPT N-values in this layer ranged from 14 to 30 blows per foot and the average was 22 blows per foot.

The moisture content of one selected sample from this layer was 22.6 percent. The liquid limit of the selected sample was 48 and the plasticity index was 22. The USCS symbol for the soil in this layer was found to be CL and the AASHTO soil classification symbol was found to be A-7-6 based on laboratory test results and visual classifications.

### **Residual Soil**

Residual soils were encountered underlying the Existing Fill in Boring BT-13 and underlying the Sandy Clay in Boring BT-14. The depth to the bottom of this layer was more than 20 feet (i.e., exceeded the boring depth). The Residual Soil consisted of medium dense to very dense, silty SAND. The SPT N-values in this layer ranged from 17 to 53 blows per foot and the average was 36 blows per foot.

The moisture contents of selected soil samples collected from this layer ranged from 12.5 to 25.9 percent. The liquid limit of two selected samples from this layer was 48 and

the plasticity indexes were 15 and 17. The USCS symbol for the soil in this layer was found to be SM, and the AASHTO soil classification symbols were found to include A-2-4, A-2-7 and A-7-5 based on the laboratory test results and visual classifications.

## Water Level Observations in the Boreholes

Water level observations were generally made in each boring upon completion and after 24 hours. The measured water level depths are noted on the boring logs and are presented in Table 1.

Table 1: Summary of Water Level Observations in the Boreholes

Boring No.	Ground Surface Elev.	Measurements at Completion		Measurements after 24 hours	
		Water Level Depth (feet)	Water Level Elev.	Water Level Depth (feet)	Water Level Elev.
BT-13	482	Dry @ 19.0'	Dry @ 463.0'	Dry @ 19.0'	Dry @ 463.0'
BT-14	494	Dry @ 20.0'	Dry @ 474.0'	Dry @ 20.0'	Dry @ 474.0'

The water level depth observations are an indication of the groundwater level at the time of the investigation. Be advised that fluctuations in groundwater levels may occur due to variations in season, rainfall, construction activity, and other site-specific factors.

## CONCLUSIONS AND RECOMMENDATIONS

According to the drawings provided by Gannett Fleming, the proposed improvements include the construction of generator and substation buildings. A new concrete retaining wall will be constructed around the east and south sides of the generator building.

### Generator Building

The proposed generator building will be constructed adjacent to the existing underground sand filter. The proposed generator building will house a new standby diesel generator and switchgear equipment, and includes an adjacent retaining wall. The proposed building will include a slab-on-grade and the walls will be supported on spread footings. The building will be approximately 45 feet long and 24 feet wide and the proposed finished floor elevation will be 503 (feet). It is anticipated that the bottom of the footings will be placed at least 2.5 feet below the final exterior grade, which will correspond to an elevation of about 500 (feet). Boring BT-14 was performed in the vicinity of the proposed generator building as well as previous Borings BT-1, BT-2 and BT-12.

Based on the drawings provided by Gannett Fleming, the proposed generator building will be located at the site of a former reservoir embankment. The embankment was excavated during the construction of the new finished water reservoir and subsequently, onsite soils were stockpiled. An existing 18-inch storm drain that crosses the proposed building site will be relocated.

Our analysis included an evaluation of the soil support for the proposed spread footings. Based on the subsurface conditions found in the borings performed in the vicinity of the proposed generator building, it appears that the footings will bear on the Existing Fill. The Existing Fill is not suitable for support of the footings due to its variable consistency and potential for excessive settlement. Therefore, it is recommended that the Existing Fill below the proposed footings be removed and replaced with compacted structural fill. Appendix F contains a *Footing Subgrade Modification Detail*, depicting the soil removal/replacement technique. As indicated in the detail, the removal of the Existing Fill should extend to a depth below each footing equal to at least two times the footing width, and should extend laterally from all sides of each footing a distance equal to the footing width.

The bottom of the Existing Fill encountered in Borings BT-1, BT-2, BT-12 and BT-14 ranged from elevation 487 to 494 (feet).

The building and retaining walls may be supported by spread footings founded on subgrades that have been modified as described above. The net allowable bearing capacity will be 2,000 psf after the subgrade has been modified. The total settlement of the footing is estimated to be 1.0 inch or less and differential settlement is not expected to exceed 0.5 inch.

It is recommended that the removal of the Existing Fill and replacement with structural fill be observed by a geotechnical engineer and/or their designated representative during construction. Test pit excavations and penetrometer testing will be needed to assess the extent of the Existing Fill within the area of the building at the time of construction. Thus, it would be prudent to include contingencies in the construction estimate/budget to account for these conditions.

Groundwater was not encountered in Boring BT-14. However, groundwater was encountered in previously drilled Borings BT-1, BT-2 and BT-12 during drilling at depths ranging from 18.8 to 40.5 feet. These depths correspond to elevations that range from 472 to 479.5 (feet). It is anticipated that the bottom of footing will be at about elevation 500 (feet) and the excavation of Existing Fill will extend to about elevation 492 (feet). Therefore, it is expected that groundwater will not be encountered in the excavations.

## **Substation Building**

The proposed substation building will be constructed between pumping stations no.2 and no.3. The proposed substation building will house new switchgear equipment and two transformers. The proposed building will include a slab-on-grade and the walls will



be supported on spread footings. The building will be approximately 59 feet long and 27.5 feet wide. The proposed finished floor elevation will be 486 (feet). It is anticipated that the bottom of the footings will be placed at least 2.5 feet below the final exterior grade, which will correspond to about elevation 483 (feet). Boring BT-13 was performed at the location of the proposed substation building.

Based on the drawings provided by Gannett Fleming, there are existing underground utilities at the location of proposed substation building that include a ductbank, 2-inch waterline, fire hydrant and 12-inch storm drain. These utilities will be relocated prior to construction of the building.

Our analysis included an evaluation of the soil support for the proposed spread footings. Based on the subsurface conditions found in Boring BT-13, it appears that the footings will bear on the Existing Fill. The Existing Fill is not suitable for support of the footings due to its variable consistency and potential for excessive settlement. Therefore, it is recommended that the Existing Fill below the proposed footings be removed and replaced with compacted structural fill. Appendix F contains a *Footing Subgrade Modification Detail*, depicting the soil removal/replacement technique. As indicated in the detail, the removal of the Existing Fill should extend to a depth below each footing equal to at least two times the footing width, and should extend laterally from all sides of each footing a distance equal to the footing width.

The bottom elevation of the Existing Fill in Boring BT-13 was 473.5 (feet).

The building foundations may be supported by spread footings founded on subgrades that have been modified as described above. The net allowable bearing capacity will be 2,000 psf after the subgrade has been modified. The total settlement of the footing is estimated to be 1.0 inch or less and differential settlement is not expected to exceed 0.5 inch.

It is recommended that the removal of the Existing Fill and replacement with structural fill be observed by a geotechnical engineer and/or their designated representative during construction. Test pit excavations and penetrometer testing will be needed to assess the extent of the Existing Fill within the area of the building at the time of construction. Thus, it would be prudent to include contingencies in the construction estimate/budget to account for these conditions.

Groundwater was not encountered in Boring BT-13, which extended to an elevation of approximately 462 (feet). The bottom of footing will be at an elevation 483 (feet) and the excavation of Existing Fill will extend to an elevation of about 475 (feet). Therefore, it is expected that groundwater will not be encountered in the excavations.

## **Slabs-on-Grade**

The slabs-on-grade for the generator and substation buildings are also expected to bear on the Existing Fill. Slabs-on-grade should not be founded on soft/loose soils. Thus, the

subgrades should be thoroughly proofrolled with a fully loaded tandem axle dump truck or equivalent prior to constructing the slabs. The purpose of the proofrolling is to locate any soft, loose or otherwise unsuitable pockets of soils. Unsuitable subgrade soils should be removed and replaced with structural fill. It is recommended that the prepared slab subgrades be evaluated by a geotechnical engineer or designated representative immediately prior to stone and concrete placement. This evaluation may include a combination of visual observations, proofrolling, hand-rod probing, and field density tests to verify that the subgrade soils have been prepared properly.

The concrete slabs-on-grade should be reinforced with steel reinforcing bars or welded wire fabric. A layer of at least 4 inches of ASTM C33, Size No. 57 stone should be provided beneath the slabs as a drainage layer. A polyethylene vapor barrier should be placed over the stone prior to concrete placement. At least 12 inches of compacted structural fill should be placed below the drainage layer for the slabs-on-grade. The recommended modulus of subgrade reaction ( $k_s$ ) for the design of the proposed slabs is 90 pounds per cubic inch (pci).

## Retaining Wall

The retaining wall at the generator building should be designed to support the lateral earth pressures imposed by backfill placed against the wall, hydrostatic pressures and appropriate surcharge loads. Based on the assumption that the fill and backfill recommendations contained in this report are followed during construction, the lateral earth pressures may be calculated using the parameters presented below.

Moist Soil Unit Weight = 125 pcf  
Effective Soil Friction Angle ( $\phi$ ) = 30 degrees  
Cohesion = 0 psf

At-rest earth pressure values should be used for conditions that do not allow movement of a wall away from the soil mass. The at-rest earth pressure coefficient,  $K_o$ , should be calculated as  $K_o = 1 - \sin \phi$ .

Active earth pressure values should be used when conditions allow movement of the walls away from the soil mass. The active earth pressure coefficient,  $K_a$ , should be calculated as  $K_a = \tan^2(45 - \phi/2)$ .

Passive earth pressures occur when a buried structural element imposes a horizontal load on the soil mass. Typically, the coefficient of passive earth pressure,  $K_p$ , is calculated as  $K_p = \tan^2(45 + \phi/2)$ .

The equivalent fluid pressures on the walls,  $p$ , may be calculated by the following equation,  $p = K\sigma_v$ ; where,  $K$  is the earth pressure coefficient and  $\sigma_v$  is the vertical overburden stress at depth,  $z$  ( $z$  is the depth below grade behind the wall or the depth

below the toe line in front of the wall). When the soil is in the active state,  $K = K_a$ ; when the soil is in the at-rest state,  $K = K_o$ ; and when the soil is in the passive state,  $K = K_p$ .

The horizontal friction coefficient between the natural soils or compacted fill and concrete foundations is estimated to be 0.35. The friction coefficient between compacted backfill and the concrete foundation walls is estimated to be 0.35.

A drainage system should be installed just above the footing against the back of the retaining wall. The drainage system should consist of 4-inch diameter perforated pipe encased in 4 inches of ASTM C33 Size No. 7 coarse aggregate and wrapped with a permeable geotextile fabric, such as MIRAFI 140N. The drainage system will serve to prevent the build-up of hydrostatic pressure behind the wall. The drainage system should be designed to drain by gravity to a storm water system or to the final grade.

## Groundwater Control

Boring BT-14 and previous Borings BT-1, BT-2 and BT-12 were performed in the vicinity of the proposed generator building. Groundwater was not encountered in Boring BT-14. However, groundwater was encountered in previously drilled Borings BT-1, BT-2 and BT-12 at elevations that ranged from 472 to 479.5 (feet). It is anticipated that the excavation of Existing Fill at the proposed generator building site will extend to about elevation 492 (feet). Therefore, it is expected that groundwater will not be encountered in the excavations.

Boring BT-13 was performed at the location of the proposed substation building. Groundwater was not encountered in Boring BT-13 which extended to an elevation of approximately 462 (feet). The bottom of footing will be at an elevation of 483 (feet) and the excavation of Existing Fill will extend to an elevation of about 475 (feet). Therefore, it is expected that groundwater will not be encountered in the excavations at the proposed substation building.

Dewatering will likely be limited to removing stormwater runoff that flows into the excavations. Commonly used dewatering techniques include pumping from sumps within the excavations.

## Fill and Backfill

All fill and backfill should be free of organic matter, muck, trash, debris, frozen material and particles larger than 3 inches.

**Structural fill and backfill:** Structural fill and backfill should be placed within the building areas, paved areas and within 5 feet of all structures. Structural fill and backfill should consist of materials classified as A-1-a, A-1-b, A-2-4, and A-3, in accordance with AASHTO M 145. The materials should have a liquid limit and plasticity index less than 40 and 10, respectively. In addition, the maximum dry density, determined in accordance with AASHTO T-180, should be no less than 110 pcf.

Structural fill and backfill should be placed in horizontal layers not more than 8 inches thick. Each layer should be compacted to no less than 97% of the maximum dry density of the soil determined in accordance with AASHTO T-180.

**Common fill and backfill:** Common fill and backfill may be used in unpaved areas more than 5 feet away from structures. Common fill and backfill should consist of soils classified as A-1-a, A-1-b, A-2-4, A-2-5, A-2-6, A-2-7, A-3, A-4, A-5 and A-6 in accordance with AASHTO M 145. In addition, the maximum dry density, determined in accordance with AASHTO T-180, should be no less than 100 pcf.

Common fill and backfill should be placed in horizontal layers not more than 8 inches thick. Common fill and backfill should be compacted to no less 92% of the maximum dry density determined in accordance with AASHTO T-180.

The soils encountered in the borings classified as A-1-b and A-2-4 are expected to be suitable for use as structural fill and backfill. The soils encountered in the borings classified as A-1-b, A-2-4, A-2-7, A-4 and A-6 are expected to be suitable for use as common fill and backfill.

Each lift of fill and backfill should be within 2 percent of the optimum moisture content prior to compaction. Each layer of fill and backfill should be compacted to the specified density before the placement of subsequent lifts.

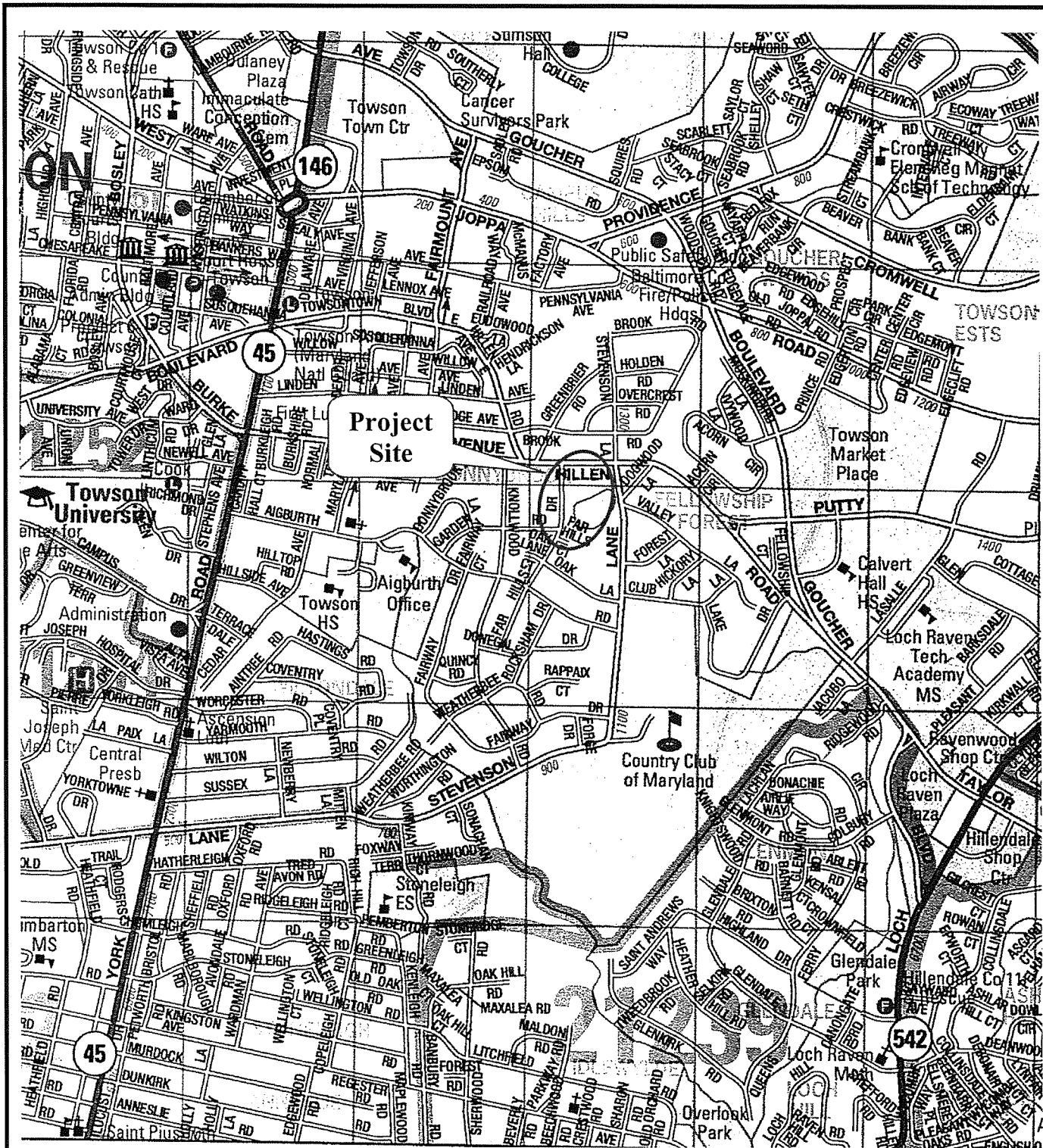
## **Inspection and Testing**

All earthwork and foundation construction should be inspected by a testing agency experienced in similar work. The testing agency should inspect the foundation and slab-on-grade subgrades, perform field density (i.e., compaction) tests on each layer of backfill, perform bearing capacity verification tests (i.e., penetrometer tests) and conduct laboratory tests as necessary to approve backfill materials. All inspection and testing should be performed under the supervision of a registered professional engineer experienced in geotechnical engineering.

## **LIMITATIONS**

The conclusions and recommendations presented in this report are based on the data collected during the geotechnical investigation. Variations in the subsurface conditions may be discovered during construction. EBA will be available to assist in determining a solution to any geotechnical problem that may arise during the construction of this project.

**Appendix A**  
**Project Vicinity Map**



**EBA**  
**EBA ENGINEERING, INC.**  
 4813 Seton Drive  
 Baltimore, Maryland 21215

Project Name:

**TOWSON FINISHED WATER  
 RESERVOIR - GENERATOR  
 & SUBSTATION BUILDINGS**

**BALTIMORE, MD**

**Project Vicinity Map**

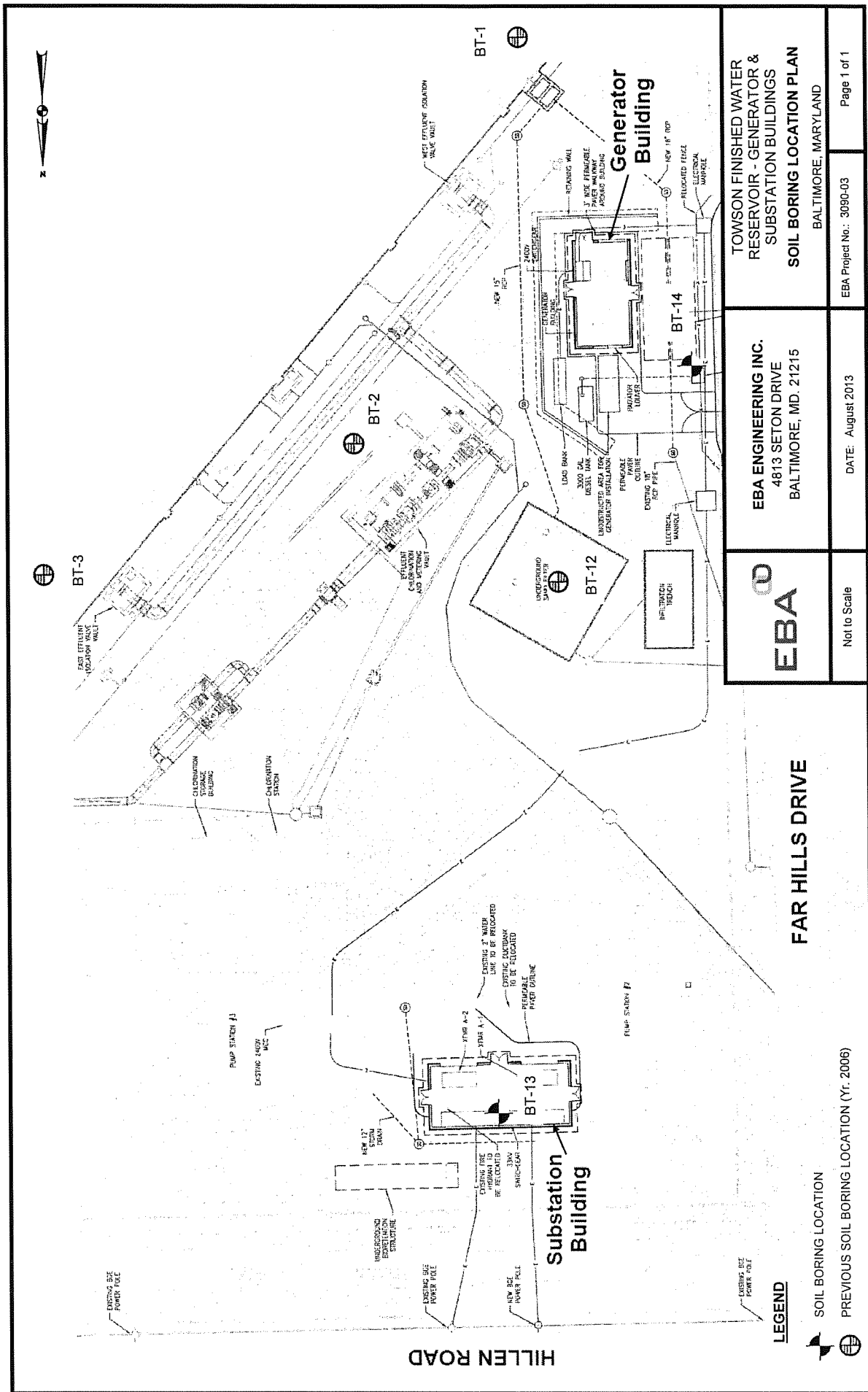
Date: 8/6/13  
 Job No.: 3090-03-002

Prepared by: GCB

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Contract No.20203 WX0  
 Addendum No.6  
 October 27, 2023

***Appendix B***  
***Soil Boring Location Plan***



<b>EBA</b>	<b>EBA ENGINEERING INC.</b> 4813 SETON DRIVE BALTIMORE, MD. 21215	<b>TOWSON FINISHED WATER          RESERVOIR - GENERATOR &amp;          SUBSTATION BUILDINGS          SOIL BORING LOCATION PLAN</b> BALTIMORE, MARYLAND
Not to Scale	DATE: August 2013	EBA Project No.: 3090-03
		Page 1 of 1



***Appendix C***  
***Boring Logs***

# 

**Project:** Towson Finished Water Reservoir - Generator & Substation Bldgs

**Boring Number:** BT-13

**Location:** Baltimore, MD

**Drilling Company:** MDA Drilling Inc.

**Job Number:** 3090-03

**Driller:** Duane Addison

**Inspector:** Girish Bhatt

**Date Drilled:** 08-01-13 & 08-01-13

**Boring Method:** HSA

**Surface Elevation:** 482' (est)

**Hole Diameter:** 8"

**Hammer Weight/Drop:** 140 lb/30 in

**Water Level at Completion:** Dry @ 19.0'


**Northing:** N/A

**Water Level After 24 hrs:** Dry @ 19.0'

**Easting:** N/A

RECORD OF SUBSURFACE EXPLORATION - 3090-03 TOWSON FINISHED WATER RESERVOIR - ADDITIONAL WORK GPJ EBA ENGINEERING INC.GDT 8/23/13

Elevation (ft)	Water/Caved Depth (ft)	Description	Depth (ft)	Sampler Number	Blows/6"	Recovery (inches)	Remarks
482			0				5 inches of topsoil.
481		Brown, moist, very stiff, sandy SILT, trace gravel and roots (Fill).		S-1	5 - 7 - 11	16	
480							
479							
478		Reddish brown / gray, moist, medium stiff, sandy CLAY (Fill).		S-2	3 - 2 - 3	10	
477			5				
476		Reddish brown / light brown, moist, medium dense, silty SAND (Fill).		S-3	2 - 7 - 5	7	
475							
474							
473		Reddish brown / gray, moist, medium dense, silty SAND (Residual Soil).		S-4	3 - 7 - 10	18	
472			10				
471							
470							
469							
468		Gray / white / pink, moist, dense, silty SAND (Residual Soil).		S-5	7 - 13 - 23	18	
467			15				
466							
465							
464							
463		Dark brown / light brown, moist, dense, silty SAND (Residual Soil).		S-6	5 - 14 - 28	18	
462			20				Installed temporary pipe @ 19.0 feet.
461		Bottom of Boring @ 20.0 ft					
460							
459							
458							
457			25				
456							
455							
454							
453							

**EBA**  **EBA Engineering, Inc.**  
4813 Seton Drive  
Baltimore, Maryland 21215

■ = Split Spoon

▽ Water Level At Completion  
▽ Water Level After 24 hrs  
⏏ Caved Depth At Completion  
⏏ Caved Depth After 24 hrs

Sheet: 1 of 1

Contract No.20203 WX0  
Addendum No.6  
October 27, 2023

# 

**Project:** Towson Finished Water Reservoir - Generator & Substation Bldgs

**Boring Number:** BT-14

**Location:** Baltimore, MD

**Drilling Company:** MDA Drilling Inc.

**Job Number:** 3090-03

**Driller:** Duane Addison

**Inspector:** Girish Bhatt

**Date Drilled:** 08-01-13 & 08-01-13

**Boring Method:** HSA

**Surface Elevation:** 494' (est)

**Hole Diameter:** 8"

**Hammer Weight/Drop:** 140 lb/30 in

**Water Level at Completion:** Dry @ 20.0'

**Northing:** N/A

**Water Level After 24 hrs:** Dry @ 20.0'

**Easting:** N/A

Elevation (ft)	Water/Caved Depth (ft)	Description	Depth (ft)	Sampler Number	Blows/6"	Recovery (inches)	Remarks
494			0				No topsoil, construction site.
493		Gray, dry, very dense, GRAVEL, trace sand (Fill).		S-1	27 - 37 - 23	12	
492							
491							
490		Gray / dark brown, dry, very dense, GRAVEL, with sand, trace roots and plastic pieces (Fill).		S-2	50/1"	1	
489			5				
488		Light brown / white, moist, very stiff, sandy CLAY.		S-3	20 - 15 - 15	15	
487							
486							
485		Light brown / white, moist, stiff, sandy CLAY.		S-4	7 - 7 - 7	2	
484			10				
483							
482							
481							
480		Reddish brown / gray, moist, very dense, silty SAND (Residual Soil).		S-5	23 - 23 - 30	3	Two attempts.
479			15				
478							
477							
476							
475		Reddish brown / gray, moist, medium dense, silty SAND (Residual Soil).		S-6	30 - 14 - 16	12	
474			20				Installed temporary pipe @ 20.0 feet.
473		Bottom of Boring @ 20.0 ft					
472							
471							
470							
469			25				
468							
467							
466							
465							

**EBA Engineering, Inc.**  
4813 Seton Drive  
Baltimore, Maryland 21215

■ = Split Spoon

▽ Water Level At Completion  
▼ Water Level After 24 hrs  
⌊ Caved Depth At Completion  
⌋ Caved Depth After 24 hrs

Sheet: 1 of 1

Contract No.20203 WX0  
Addendum No.6  
October 27, 2023

***Appendix D***  
***Laboratory Test Results***

**TOWSON FINISHED WATER RESERVOIR – GENERATOR SUBSTATION BUILDINGS**  
**Baltimore, Maryland**  
**SUMMARY OF LABORATORY TEST RESULTS**

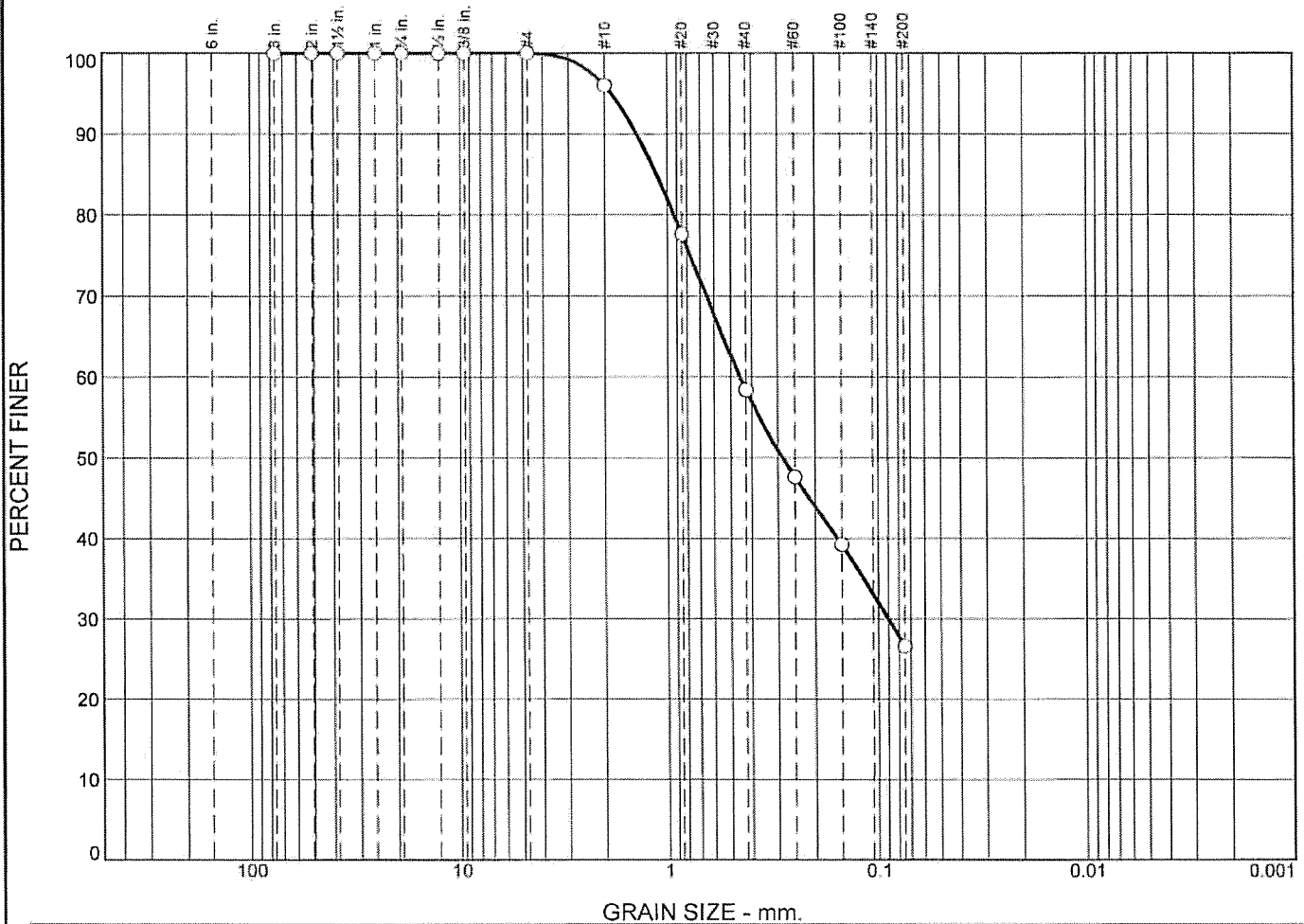
Boring No.	Sample No.	Depth (ft)	Moisture Content %	Atterberg Limits (% <sub>a</sub> )			Grain Size Analysis %			Soil Classification	
				LL	PL	PI	Gravel	Sand	Fines	USCS	AASHTO
BT-13	S-2	3.5-5.0	20.0								
	S-4	8.5-10.0	12.5	---	---	---	0.0	73.4	26.6	SM	A-2-4
	S-5	13.5-15.0	19.8	---	---	---	0.3	67.3	32.4	SM	A-2-4
	S-6	18.5-20.0	25.9	48	17	17	0.4	70.8	28.8	SM	A-2-7
BT-14	S-3	6.0-7.5	22.6	48	22	22	0.5	30.1	69.4	CL	A-7-6
	S-6	18.5-20.0	24.6	48	15	15	0.3	54.9	44.8	SM	A-7-5

LL: Liquid Limit	USCS: Unified Soil Classification System
PI: Plasticity Index	AASHTO: American Assoc. of State Highway and Transportation Officials



EBA Engineering Inc.

# Particle Size Distribution Report



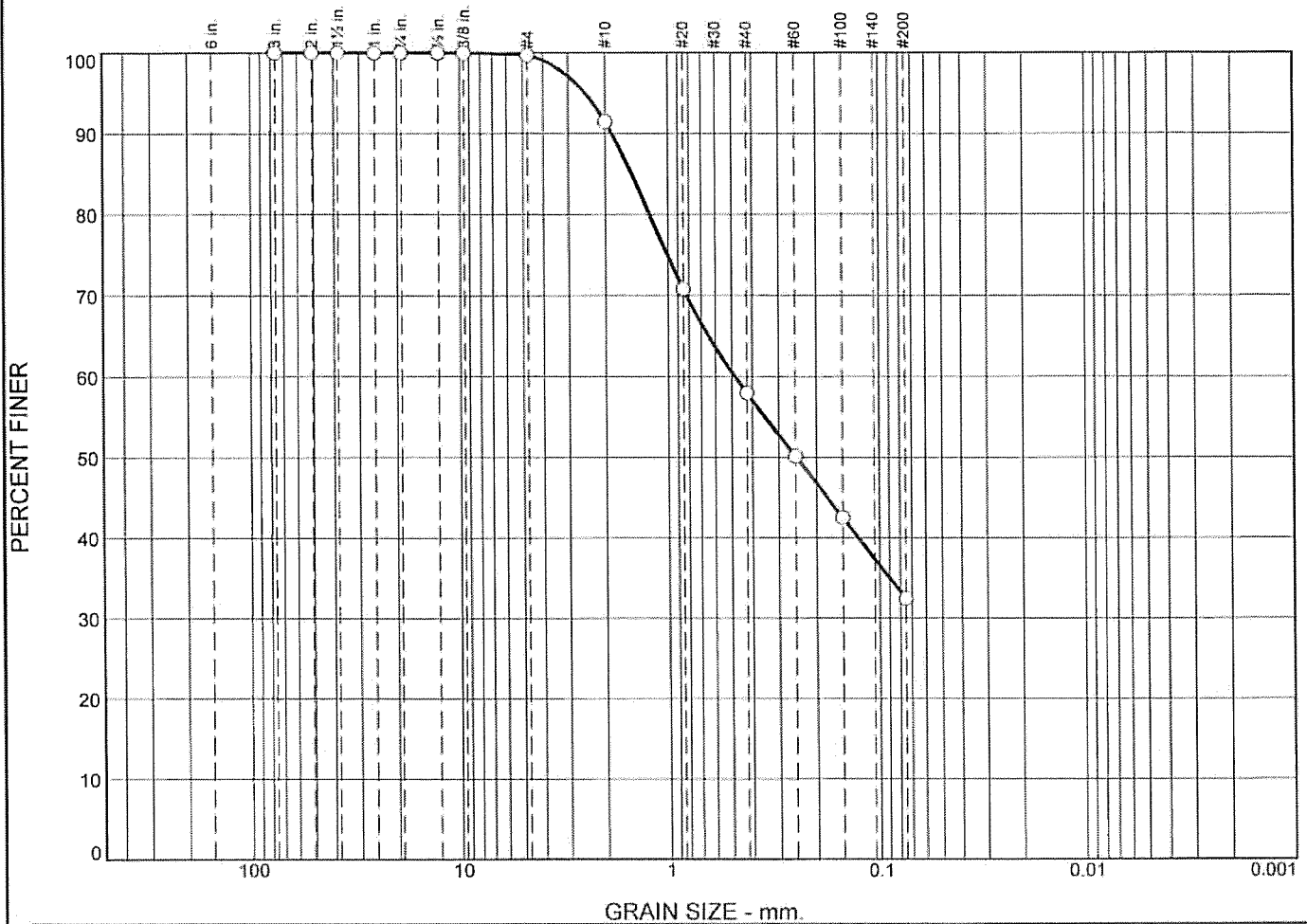
GRAIN SIZE - mm.										
% +3"		% Gravel		% Sand			% Silt		% Clay	
○	0.0		0.0		73.4			26.6		
⊗	LL	PL	D <sub>85</sub>	D <sub>60</sub>	D <sub>50</sub>	D <sub>30</sub>	D <sub>15</sub>	D <sub>10</sub>	C <sub>c</sub>	C <sub>u</sub>
○			1.1249	0.4533	0.2854	0.0899				

Material Description							USCS	AASHTO
Reddish brown / gray silty sand							SM	A-2-4

<b>Project No.</b> 3090-03-002 <b>Client:</b> Gannett Fleming, Inc. <b>Project:</b> Towson FWR-Generator & Substation Bldgs, Baltimore, MD  ○ <b>Source of Sample:</b> BT-13 <b>Depth:</b> 8.5'-10.0' <b>Sample Number:</b> S-4	<b>Remarks:</b> ○ Visual classification in accordance with ASTM D2488 Moisture content = 12.5% Date tested: 08/2013
<div>EBA Engineering, Inc.</div> <div>Baltimore, MD</div>	

Tested By: SB                      Checked By: SK

# Particle Size Distribution Report



GRAIN SIZE - mm.										
% +3"		% Gravel		% Sand			% Silt		% Clay	
○	0.0		0.3		67.3			32.4		
×	LL	PL	D <sub>85</sub>	D <sub>60</sub>	D <sub>50</sub>	D <sub>30</sub>	D <sub>15</sub>	D <sub>10</sub>	C <sub>c</sub>	C <sub>u</sub>
○			1.4905	0.4843	0.2477					

Material Description							USCS	AASHTO
Gray / white / pink silty sand							SM	A-2-4

Project No. 3090-03-002 Client: Gannett Fleming, Inc.  
 Project: Towson FWR-Generator & Substation Bldgs, Baltimore, MD

Source of Sample: BT-13 Depth: 13.5'-15.0' Sample Number: S-5

EBA Engineering, Inc.  
 Baltimore, MD

## Remarks:

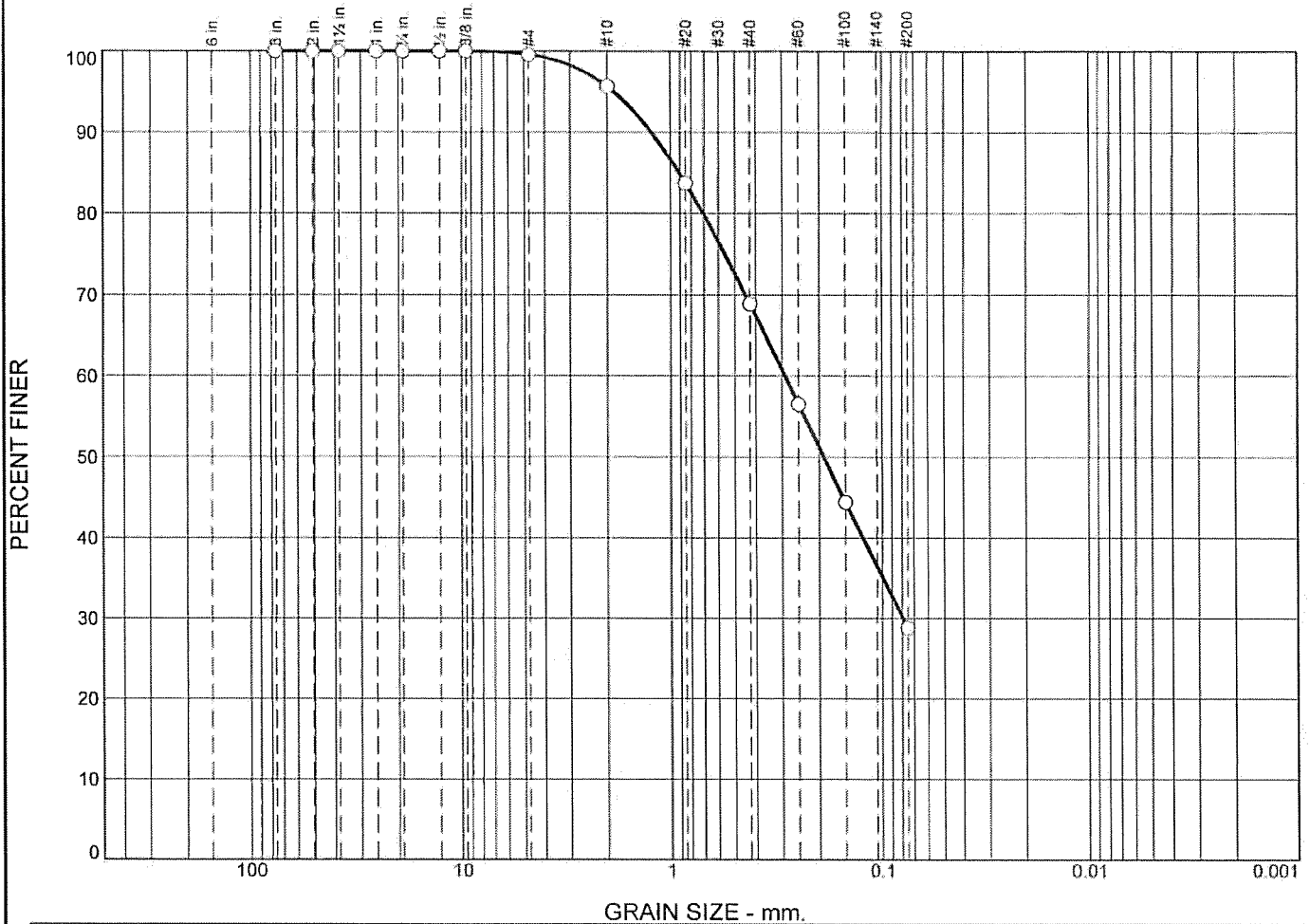
Visual classification in accordance with ASTM D2488  
 Moisture content = 19.8%  
 Date tested: 08/2013

Figure

Tested By: SB

Checked By: SK

# Particle Size Distribution Report



GRAIN SIZE - mm.										
% +3"		% Gravel		% Sand			% Silt		% Clay	
○	0.0		0.4		70.8			28.8		
⊗	LL	PL	D <sub>85</sub>	D <sub>60</sub>	D <sub>50</sub>	D <sub>30</sub>	D <sub>15</sub>	D <sub>10</sub>	C <sub>c</sub>	C <sub>u</sub>
○	48	31	0.9087	0.2899	0.1902	0.0792				

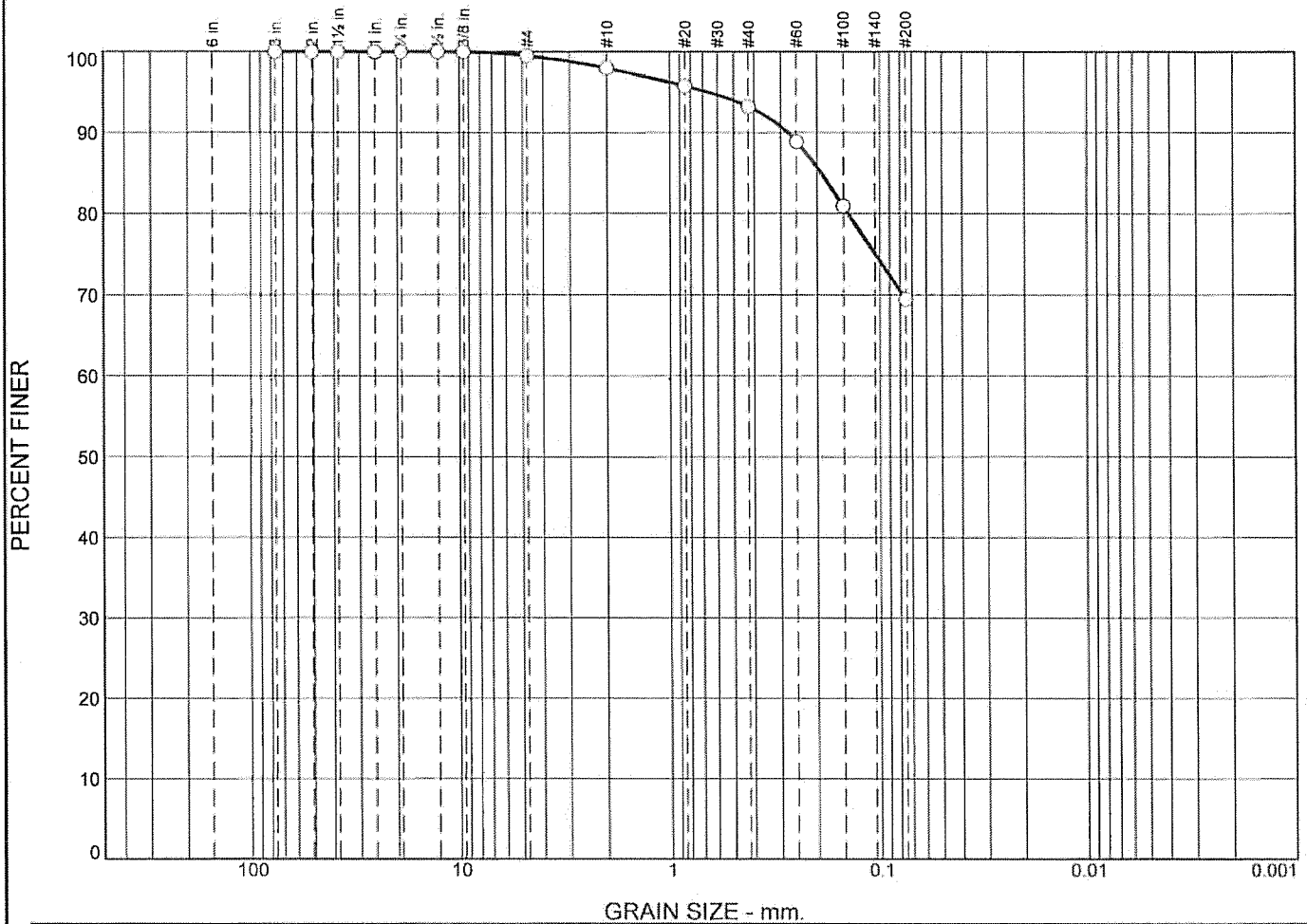
Material Description							USCS	AASHTO
○ Dark brown / light brown silty sand							SM	A-2-7(1)

<b>Project No.</b> 3090-03-002 <b>Client:</b> Gannett Fleming, Inc. <b>Project:</b> Towson FWR-Generator & Substation Bldgs, Baltimore, MD  ○ <b>Source of Sample:</b> BT-13 <b>Depth:</b> 18.5'-20.0' <b>Sample Number:</b> S-6	<b>Remarks:</b> ○ Moisture content = 25.9% Date tested: 08/2013
<b>EBA Engineering, Inc.</b>  <b>Baltimore, MD</b>	

Tested By: SB      Checked By: SK



# Particle Size Distribution Report



GRAIN SIZE - mm.

% +3"		% Gravel		% Sand			% Silt		% Clay	
○	0.0	0.5		30.1			69.4			
×	LL	PL	D <sub>85</sub>	D <sub>60</sub>	D <sub>50</sub>	D <sub>30</sub>	D <sub>15</sub>	D <sub>10</sub>	C <sub>c</sub>	C <sub>u</sub>
○	48	26	0.1907							

Material Description							USCS	AASHTO
Light brown / white sandy lean clay							CL	A-7-6(15)

Project No. 3090-03-002 Client: Gannett Fleming, Inc.  
 Project: Towson FWR-Generator & Substation Bldgs, Baltimore, MD  
 Source of Sample: BT-14 Depth: 6.0'-7.5' Sample Number: S-3

Remarks:  
 Moisture content 22.6%  
 Date tested: 08/2013

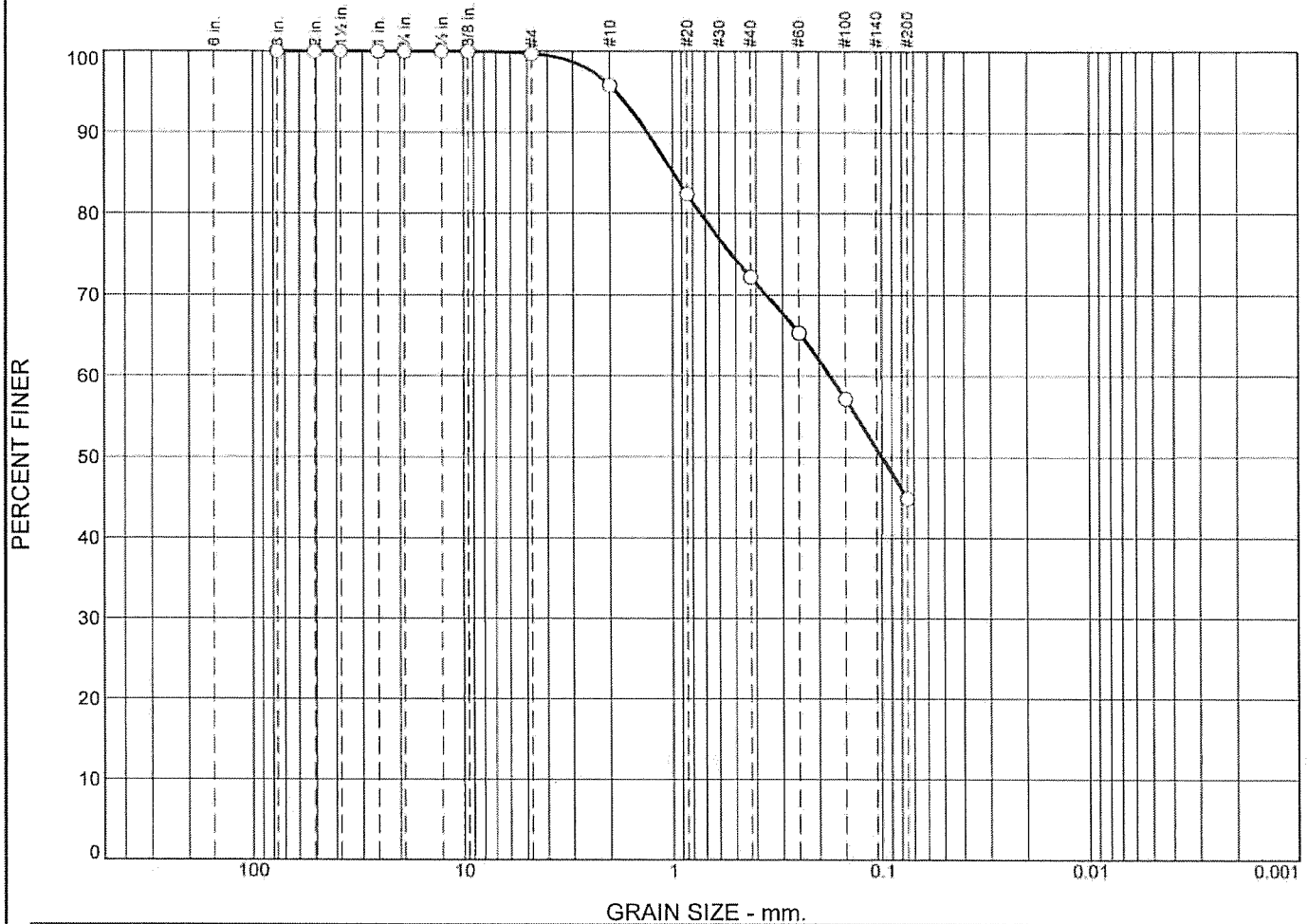
EBA Engineering, Inc.  
 Baltimore, MD

Figure

Tested By: SB

Checked By: SK

# Particle Size Distribution Report



GRAIN SIZE - mm.										
% +3"		% Gravel		% Sand			% Silt		% Clay	
○	0.0		0.3		54.9			44.8		
⊗	LL	PL	D <sub>85</sub>	D <sub>60</sub>	D <sub>50</sub>	D <sub>30</sub>	D <sub>15</sub>	D <sub>10</sub>	C <sub>c</sub>	C <sub>u</sub>
○	48	33	0.9886	0.1772	0.0998					

Material Description							USCS	AASHTO
Reddish brown / gray silty sand							SM	A-7-5(4)

<b>Project No.</b> 3090-03-002 <b>Client:</b> Gannett Fleming, Inc. <b>Project:</b> Towson FWR-Generator & Substation Bldgs, Baltimore, MD  <b>Source of Sample:</b> BT-14 <b>Depth:</b> 18.5'-20.0' <b>Sample Number:</b> S-6				<b>Remarks:</b> ○ Moisture content = 24.6% Date tested: 08/2013	
<b>EBA Engineering, Inc.</b>  <b>Baltimore, MD</b>				<b>Figure</b>	

Tested By: SB                      Checked By: SK

***Appendix E***  
***Previous Boring Logs***

# RECORD OF SUBSURFACE EXPLORATION

**Project:** Towson Finished Water Reservoir Cover

**Location:** Towson, Maryland

**Job Number:** 3090C0139

**Inspector:** Jiwei Duan

**Boring Method:** Hollow Stem Auger

**Hole Diameter:** 6"

**Water Level at Completion:** 44.4', caved @ 47.1'

**Boring Number:** BT-1

**Drilling Company:** EBA Engineering, Inc.

**Driller:** Duane Addison

**Date Drilled:** 01-20-06

**Surface Elevation:** 512.5' (est)

**Hammer Weight/Drop:** 140 Lbs/30 in.

**Water Level After 24 hrs:** 40.5'

RECORD OF SUBSURFACE EXPLORATION TOWSON FINISHED WATER RESERVOIR COVERS BORINGS.GPJ EBA ENGINEERING INC.GDT 3/18/06

Elevation (ft)	Water/Caved Depth (ft)	Description	Depth (ft)	Sampler Number	Blows/6"	Recovery (inches)	Remarks
512.5			0				3" topsoil.
512		Brown / reddish brown, moist, loose, silty sand with gravel (fill).		S1	5 - 3 - 4	3	
511							
510							
509							
509		Brown / brownish red, moist, stiff, sandy CLAY, trace gravel (fill).		S2	3 - 3 - 7	12	
508							
507							
506							
506		Brown / brownish red, moist, soft, sandy CLAY, little gravel (fill).		S3	2 - 2 - 2	6	
505							
504							
503							
503		Brown / brownish red, moist, soft, sandy CLAY (fill). Dark brown, wet, very loose, SAND, trace silt (fill).		S4A S4B	2 - 1 - 1	16	
502							
501							
500							
499		Brown, wet, very loose, clayey SAND (fill)		S5	1 - 2 - 2	4	
498							
497							
496							
494		Light yellow / pink, moist, stiff, sandy SILT.		S6	3 - 7 - 6	18	
493							
492							
491							
489		Pink / white / mottled, moist, stiff, sandy SILT.		S7	3 - 3 - 8	18	
488							
487							
486							
484		Pink / white / mottled, moist, stiff, sandy SILT.		S8	3 - 5 - 5	18	
483							
482							
481							
480		Mottled, brown, gray, moist, medium dense, silty SAND.		S9	15 - 9 - 6	12	
479							
478							
478							



**EBA Engineering, Inc.**  
**4813 Seton Drive**  
**Baltimore, Maryland 21215**

= Standard Penetration Test

Water Level At Completion  
 Water Level After 24 hrs  
 Caved Depth At Completion  
 Caved Depth After 24 hrs

Sheet: 1 of 2

# RECORD OF SUBSURFACE EXPLORATION

**Project:** Towson Finished Water Reservoir Cover

**Location:** Towson, Maryland

**Job Number:** 3090C0139

**Inspector:** Jiwei Duan

**Boring Method:** Hollow Stem Auger

**Hole Diameter:** 6"

**Water Level at Completion:** 44.4', caved @ 47.1'

**Boring Number:** BT-1

**Drilling Company:** EBA Engineering, Inc.

**Driller:** Duane Addison

**Date Drilled:** 01-20-06

**Surface Elevation:** 512.5' (est)

**Hammer Weight/Drop:** 140 Lbs/30 in.

**Water Level After 24 hrs:** 40.5'

Elevation (ft)	Water/Caved Depth (ft)	Description	Depth (ft)	Sampler	Number	Blows/6"	Recovery (inches)	Remarks
477.5			35					
477								
476								
475								
474								
473		Mottled brown, reddish brown, yellowish brown, white, moist, loose, silty SAND, trace clay.	40		S10	2 - 4 - 5	18	
472								
471								
470								
469								
468		Mottled brown, dark brown, gray, moist, medium dense, silty SAND, trace gravel.	45		S11	6 - 7 - 10	18	
467								
466								
465								
464								
463		Mottled brown, dark brown, gray, medium dense, silty SAND, trace gravel.	50		S12	4 - 7 - 9	18	
462								
461								
460								
459								
458		Mottled black, black, brown, gray, moist, dense, silty SAND, trace gravel.	55		S13	5 - 13 - 12	18	
457								
456								
455								
454								
453		Mottled white, gray, brown, dense, SAND, trace silt and gravel.	60		S14	7 - 13 - 15	18	
452		Bottom of Boring @ 60.0 ft						Installed temporary monitoring well @ 60.0 feet.
451								
450								
449								
448								
447								
446								
445								
444								
443								



**EBA Engineering, Inc.**  
4813 Seton Drive  
Baltimore, Maryland 21215

☐ = Standard Penetration Test

☒ Water Level At Completion  
☒ Water Level After 24 hrs  
☒ Caved Depth At Completion  
☒ Caved Depth After 24 hrs

Sheet: 2 of 2

# **RECORD OF SUBSURFACE EXPLORATION**

**Project:** Towson Finished Water Reservoir Cover

**Location:** Towson, Maryland

**Job Number:** 3090C0139

**Inspector:** Jiwei Duan

**Boring Method:** Hollow Stem Auger

**Hole Diameter:** 6"

**Water Level at Completion:** 35.3' caved @ 47.8'

**Boring Number:** BT-2

**Drilling Company:** EBA Engineering, Inc.

**Driller:** Duane Addison

**Date Drilled:** 01-19-06

**Surface Elevation:** 512' (est)

**Hammer Weight/Drop:** 140 Lbs/30 in.

**Water Level After 24 hrs:** 32.5' caved @ 43.2'

Elevation (ft)	Water/Caved Depth (ft)	Description	Depth (ft)	Sampler Number	Blows/6"	Recovery (inches)	Remarks
512			0				2" topsoil.
511		Brown, moist, loose, silty sand, some gravel (fill).		S1	4 - 5 - 5	10	
510							
509		No recovery.		S2	5 - 6 - 6	6	
508			5				
507							
506		Reddish brown, moist, very loose, clayey SAND, trace gravel (fill).		S3	2 - 2 - 2	8	
505							
504							
503		Reddish brown / dark brown, moist, very loose, clayey SAND, trace silt (fill).		S4	2 - 1 - 2	2	
502			10				
501							
500							
499							
498		Light gray / brown, moist, soft, sandy SILT with gravel (fill).		S5	2 - 2 - 2	9	
497			15				
496							
495							
494							
493		Brown / reddish brown, moist, very stiff, sandy CLAY with gravel (fill).		S6	5 - 8 - 9	11	
492			20				
491							
490							
489							
488		Brown, moist, medium dense, silty SAND, trace gravel (fill).		S7	6 - 6 - 7	6	
487			25				
486							
485							
484							
483		Mottled red / gray, moist, stiff, sandy SILT.		S8	3 - 6 - 6	18	
482			30				
481							
480							
479							
478		Mottled gray / white, moist, medium dense, silty SAND, little gravel.		S9	5 - 7 - 8	18	

SURFACE EXPLORATION TOWSON FINISHED WATER RESERVOIR COVERS BORINGS CPT EBA ENGINEERING INC. GDT 3/16/06



**EBA Engineering, Inc.**  
**4813 Seton Drive**  
**Baltimore, Maryland 21215**

☐ = Standard Penetration Test

☒ Water Level At Completion  
☒ Water Level After 24 hrs  
☒ Caved Depth At Completion  
☒ Caved Depth After 24 hrs

Sheet: 1 of 2

# 

**Project:** Towson Finished Water Reservoir Cover

**Location:** Towson, Maryland

**Job Number:** 3090C0139

**Inspector:** Jiwei Duan

**Boring Method:** Hollow Stem Auger

**Hole Diameter:** 6"

**Water Level at Completion:** 35.3', caved @ 47.8'

**Boring Number:** BT-2

**Drilling Company:** EBA Engineering, Inc.

**Driller:** Duane Addison

**Date Drilled:** 01-19-06

**Surface Elevation:** 512' (est)

**Hammer Weight/Drop:** 140 Lbs/30 in.

**Water Level After 24 hrs:** 32.5', caved @ 43.2'

Elevation (ft)	Water/Caved Depth (ft)	Description	Depth (ft)	Sampler Number	Blows/6"	Recovery (inches)	Remarks
477			35				
476							
475							
474							
473		Mottled dark gray / black / white, wet, medium dense, silty SAND, trace gravel.	40	S10	3 - 7 - 8	18	
472							
471							
470							
469							
468		Dark brown, moist, dense, silty SAND, trace gravel.	45	S11	10 - 11 - 17	18	Observed wet spoon.
467							
466							
465							
464							
463		Mottled dark brown / white, very dense, SAND with gravel.	50	S12	26 - 63 - 50	14	
462							
461							
460							
459							
458		Dark gray, moist, very dense, silty SAND, trace gravel.	55	S13	100/4"	4	
457							
456							
455							
454							
453		Dark gray, moist, very dense, SAND, trace gravel.	60	S14	100/2"	2	
452		Bottom of Boring @ 58.7 ft					
451							
450							
449							
448							
447							
446							
445							
444							
443							

RECORD OF



**EBA Engineering, Inc.**  
4813 Seton Drive  
Baltimore, Maryland 21215

□ = Standard Penetration Test

▽ Water Level At Completion

▼ Water Level After 24 hrs

⊥ Caved Depth At Completion

⊗ Caved Depth After 24 hrs

Sheet: 2 of 2

# 

**Project:** Towson Finished Water Reservoir Cover

**Location:** Towson, Maryland

**Job Number:** 3090C0139

**Inspector:** Girish Bhatt

**Boring Method:** Hollow Stem Auger

**Hole Diameter:** 6"

**Water Level at Completion:** 13.5', caved @ 31.5'

**Boring Number:** BT-12

**Drilling Company:** EBA Engineering, Inc.

**Driller:** John Accord

**Date Drilled:** 02-27-06 & 02-28-06

**Surface Elevation:** 493.5' (est)

**Hammer Weight/Drop:** 140 Lbs/30 in.

**Water Level After 24 hrs:** 18.8', caved @ 21.3'

Elevation (ft)	Water/Caved Depth (ft)	Description	Depth (ft)	Sampler Number	Blows/6"	Recovery (inches)	Remarks
493.5			0				
493							
492		Brownish red / reddish brown, moist, stiff, sandy SILT (fill).		S1	3 - 4 - 7	8	1" topsoil.
491							
490		Brownish yellow, moist, stiff, SILT, trace sand.		S2	7 - 5 - 6	12	
489			5				
488							
487		Brownish yellow / light gray, moist, sandy SILT, trace gravel.		S3	7 - 12 - 15	18	
486							
485		Light gray / light brown, moist, very stiff, sandy SILT.		S4	7 - 8 - 10	12	
484			10				
483							
482							
481							
480	▽	Light gray / pink, moist, dense, silty SAND, trace to little gravel.		S5	10 - 11 - 14	13	
479			15				
478							
477							
476							
475	▽	Gray / pink, moist, medium dense, silty SAND, trace to little gravel.		S6	13 - 11 - 13	15	
474			20				
473							
472	⊥						
471							
470		Gray / pink, moist, dense, silty SAND, trace to little gravel.		S7	10 - 21 - 27	15	
469			25				
468							
467							
466							
465		Gray / pink, moist to wet, very dense, silty SAND, little gravel.		S8	100/5"	5	Observed wet spoon.
464			30				
463							
462	⊥						
461							
460		Gray / pink, moist, very dense, SAND with gravel.		S9	100/1"	1	Auger refusal @ 34.0 feet



**EBA Engineering, Inc.**  
**4813 Seton Drive**  
**Baltimore, Maryland 21215**

□ = Standard Penetration Test  
 ■ = Rock Core

▽ Water Level At Completion  
 ▽ Water Level After 24 hrs  
 ⊥ Caved Depth At Completion  
 ⊥ Caved Depth After 24 hrs

Sheet: 1 of 2



# 

**Project:** Towson Finished Water Reservoir Cover

**Location:** Towson, Maryland

**Job Number:** 3090C0139

**Inspector:** Girish Bhatt

**Boring Method:** Hollow Stem Auger

**Hole Diameter:** 6"

**Water Level at Completion:** 13.5' caved @ 31.5'

**Boring Number:** BT-12

**Drilling Company:** EBA Engineering, Inc.

**Driller:** John Accord

**Date Drilled:** 02-27-06 & 02-28-06

**Surface Elevation:** 493.5' (est)

**Hammer Weight/Drop:** 140 Lbs/30 in.

**Water Level After 24 hrs:** 18.8' caved @ 21.3'

Elevation (ft)	Water/Caved Depth (ft)	Description	Depth (ft)	Sampler	Number	Blows/6"	Recovery (inches)	Remarks
459								
458		Gray / pink, SAND with gravel.	35		RC1		15	Installed temporary monitoring well @ 34.0 feet.
457								REC = 25%, RQD = 0%
456								
455								
454		Bottom of Boring @ 39.0 ft	40					
453								
452								
451								
450								
449								
448								
447								
446								
445								
444								
443								
442								
441								
440								
439								
438								
437								
436								
435								
434								
433								
432								
431								
430								
429								
428								
427								
426								
425								



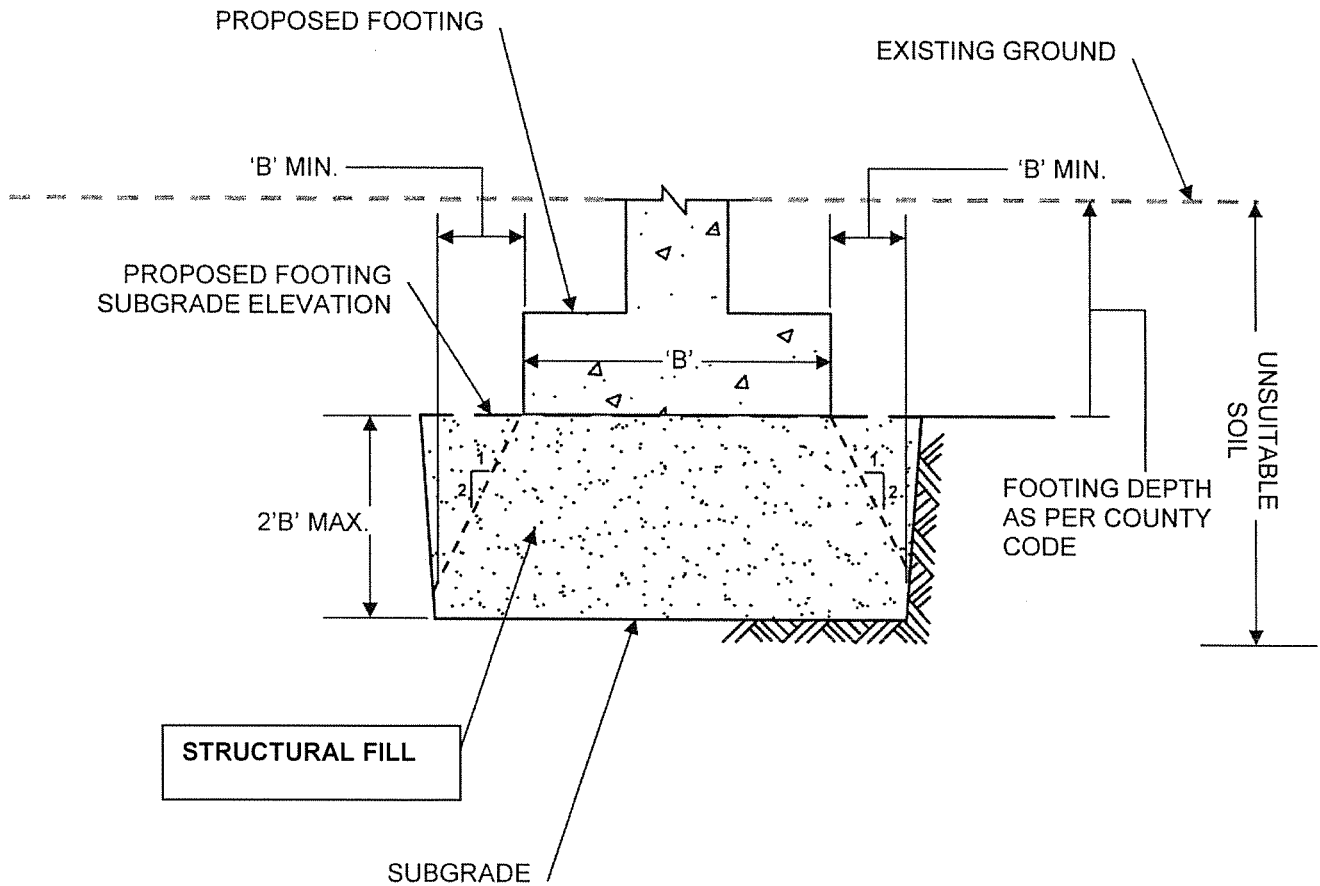
**EBA Engineering, Inc.**  
4813 Seton Drive  
Baltimore, Maryland 21215

□ = Standard Penetration Test  
■ = Rock Core

▽ Water Level At Completion  
▽ Water Level After 24 hrs  
⊥ Caved Depth At Completion  
⊥ Caved Depth After 24 hrs

Sheet: 2 of 2

***Appendix F***  
***Footing Subgrade Modification Detail***



**EBA ENGINEERING, INC.**  
4813 Seton Drive  
Baltimore, Maryland 21215

Project Name:

**TOWSON FINISHED WATER  
RESERVOIR - GENERATOR  
& SUBSTATION BUILDINGS**  
**BALTIMORE, MD**

**Figure: Footing Subgrade  
Modification Detail**

Date: 8/23/13  
Job No.: 3090-03-002

Prepared by: GCB

Not to Scale



- 
- A horizontal scale bar with alternating black and white segments. Above the bar, the numbers 20, 0, 20, and 40 are marked at regular intervals. Below the bar, the text "SCALE IN FEET" is centered.



REVISIONS BY: DHJ DATE: APRIL 2019  
THESE RECORD DRAWINGS HAVE BEEN PREPARED, IN PART, ON THE  
BASIS OF INFORMATION COMPILED BY OTHERS. THEY ARE NOT  
INTENDED TO REPRESENT IN DETAIL THE EXACT LOCATION, TYPE OF  
COMPONENT OR MANNER OF CONSTRUCTION. THE ENGINEER WILL NOT  
BE RESPONSIBLE FOR ANY ERRORS OR OMISSIONS WHICH HAVE BEEN  
INCORPORATED INTO THE DRAWINGS.

THE ORIGINAL MYLAR HAS BEEN REPLACED WITH THIS AS-BUILT MYLAR.

SHEET	68	OF	93
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REVISIONS			
NO.	DESCRIPTION	DATE	BY

